Using CFRP for Shear Strengthening of Continuous SCC Hollow Beams Containing Transverse Internal Ribs

Ali Hameed Aziz Ph.D (Asst.Prof.)* Ashraf A. Alfeehan Ph.D (Asst.Prof.)* awaz Adel Hussain(Lecturer)*

Abstract

The present paper is devoted to investigate, experimentally, the shear strength of continuous self-compacting concrete hollow beams containing internal concrete ribs and externally strengthened by strips of carbon fiber reinforced polymer (CFRP). Six full-scale beam specimens were cast and tested. The adopted variables in this study are the number of internal concrete ribs and U-shape external CFRP strips. Experimental results show that the shear failure was the dominant failure for all the tested beams. The cracking and ultimate loads are reduced by about (19.6 and 30.6%) for the hollow beam specimens which contains, five and three ribs respectively, compared with the reference beam. The ultimate load was increased by about (19.13%) for the solid beams strengthened by CFRP strips compared with the solid beam without strengthening. The ultimate load was increased by about (39%) for five internal ribs hollow beams strengthened by CFRP strips compared with the same beams but without strengthening. While, the ultimate load was increased by about (27.5%) for beam specimen which has three internal ribs with CFRP strips in comparison with the same beams but without strengthening.

Keyword: CFRP Strips, Shear, Continuous Beam, Self-Compacting Concrete, Hollow, Ribs.

* Al-Mustansireah University

1-Introduction

Rehabilitation of deteriorated structures has been a major concern in the last years. The deterioration and other factors, like reduction in steel reinforcement area due to corrosion, increase of the service load, and design/construction defects effect on shear capacity (Khalifa, et al 1999). The shear failure has different characteristics, as compared to bending failure, in which the former is more brittle and often occurs without any forewarning (Täljsten B, Elfgren L 2000). One of the techniques used to strengthen existing reinforced concrete members involves externally bonding fiber reinforced polymer (FRP) composite materials by means of epoxy adhesives. This technique, improves the structural performance of a member under ultimate load and service load (Neale 2000). The wide use of such strengthening method, for various structures such as buildings and bridges, has demonstrated its efficiency and its convenience (Bakis et al. 2002; Clarke 2000). The main objective of the present paper is to evaluate, experimentally, the shear behavior of hollow continuous, self-compacting concrete beams containing transverse internal ribs and strengthened externally with CFRP.

2-Experimental Program

Tests were carried out on six full scale solid and hollow rectangular section, continuous (resting on three supports) beam specimens under monotonic concentrated load (at the center of each span). To ensure shear failure, the beam specimens were designed with minimum shear reinforcement. The adopted variables in this study are the number of internal concrete ribs and U-shape external CFRP strips, while the type of concrete; beams dimensions, shear span-depth ratio (a/d), longitudinal and transverse reinforcement were kept constant for all the tested beams. Furthermore, a series of tests were performed on the concrete mixes; therefore, the mechanical properties of hardened and fresh concrete tests were included in this paper.

2-1-Beam Specimens Details

Six full-scale beam specimens were cast and tested in this research. The beams were continuous and made with SCC and have a section dimensions of (2500mm), (250mm) and (300mm) for length, width and height respectively. The dimensions used for the hollow core are (100mm) for width and (150mm) for height and the typical rib thickness was (100mm). It may be noted that, each beam specimen is designated in such a way to indicate the concrete type (SCC), number of internal ribs (R0 or R3 or R5) and orientation of U-shaped CFRP Strips (90º) as shown in Table (1) and Figures (1) to (6).

Table (1) Details of Tested beams.

Beam	Beam Encoding	Dimensions (mm)			No. of Internal Ribs	Orientation of U-shape CFRP Strips
		L	W	H		
B1*	SCC-R0-00				-	-
B2	SCC-R3-00				3	-
B3	SCC-R5-00				5	-
B4	SCC-R0-90º	2500	250	300	-	90º
B5	SCC-R3-90º				3	90º
B6	SCC-R5-90º				5	90º

*Reference (Solid Beam).

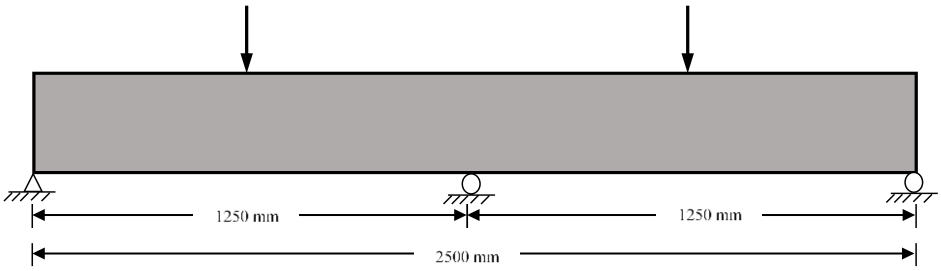

Figure (1) Description of B1 (Control Beam)

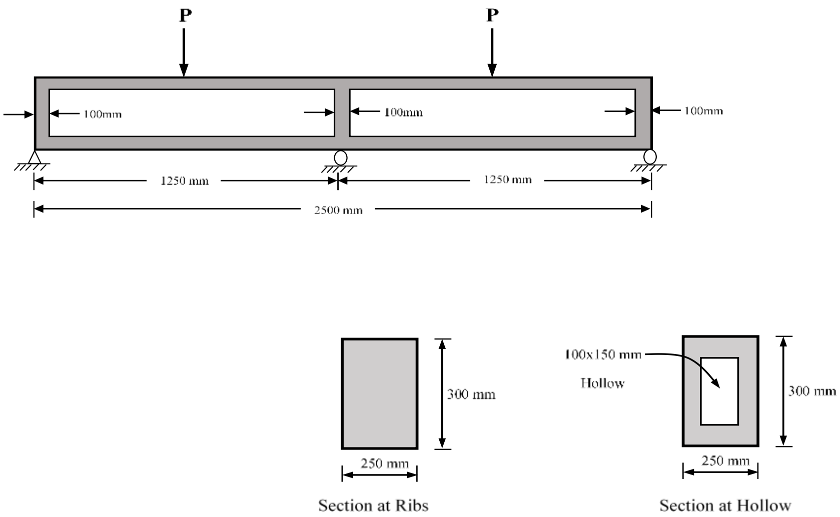

Figure (2) Description of B2 (SCC-R3-00)

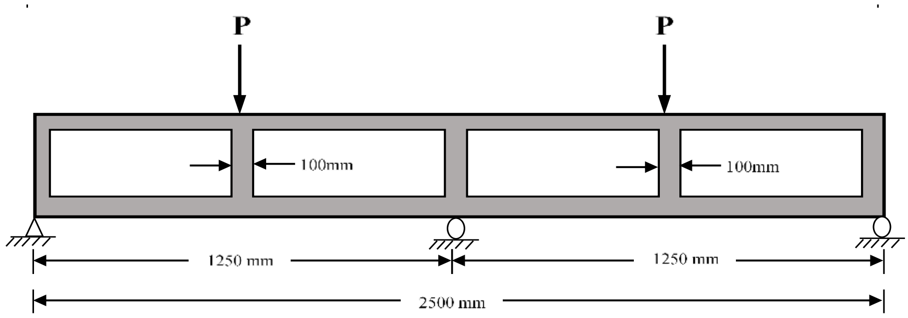

Figure (3) Description of B3 (SCC-R5-00)

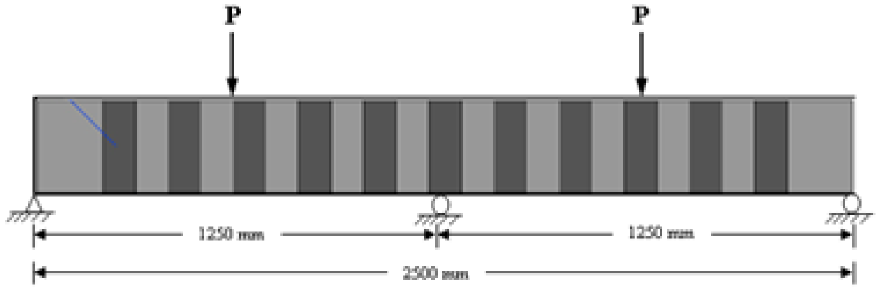

Figure (4) Description of B4 (SCC-R0-90)

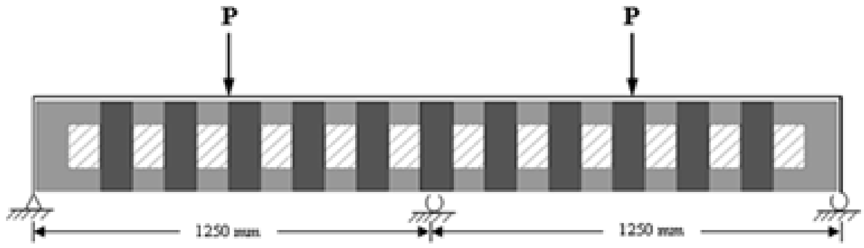

Figure (5) Description of B5 (SCC-R3-90)

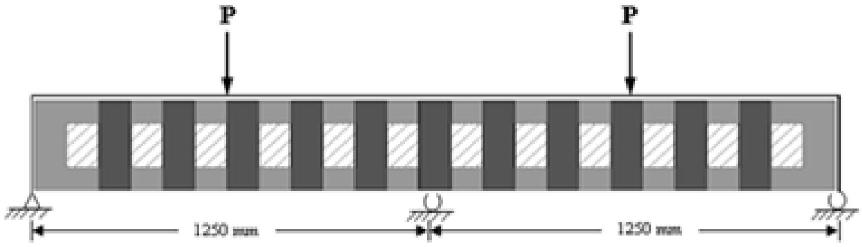

Figure (6) Description of B6 (SCC-R5-90)

2-2-Beam Specimens Reinforcement

All beam specimens have similar steel reinforcement. The flexural reinforcement consists of (4Ø20mm) at the bottom (extended throughout the beam length) and (4Ø20mm) at the top (at middle support to resist the negative moments). While, the web reinforcement (shear reinforcement), consists of (Ø8mm@130mm) as stirrups. It may be noted that, to hold the stirrups in place (2Ø6mm) at the top were used as shown in Figure (7).

To form the hollows inside the beam specimens, polystyrene blocks were used, due to its lightweight and easy to configure with the required dimensions. The dimensions of hollows were kept constant for all the hollow beams to be (100x150mm), with length of (1200mm) and (500mm) for the beams that have three and five ribs respectively, as shown in Figure (8). For beam specimens strengthened by U-shaped strips of CFRP (orientated by (90º)), Sika Wrap-300 C/60 type and epoxy based impregnating resin Sikadur-330 were used.

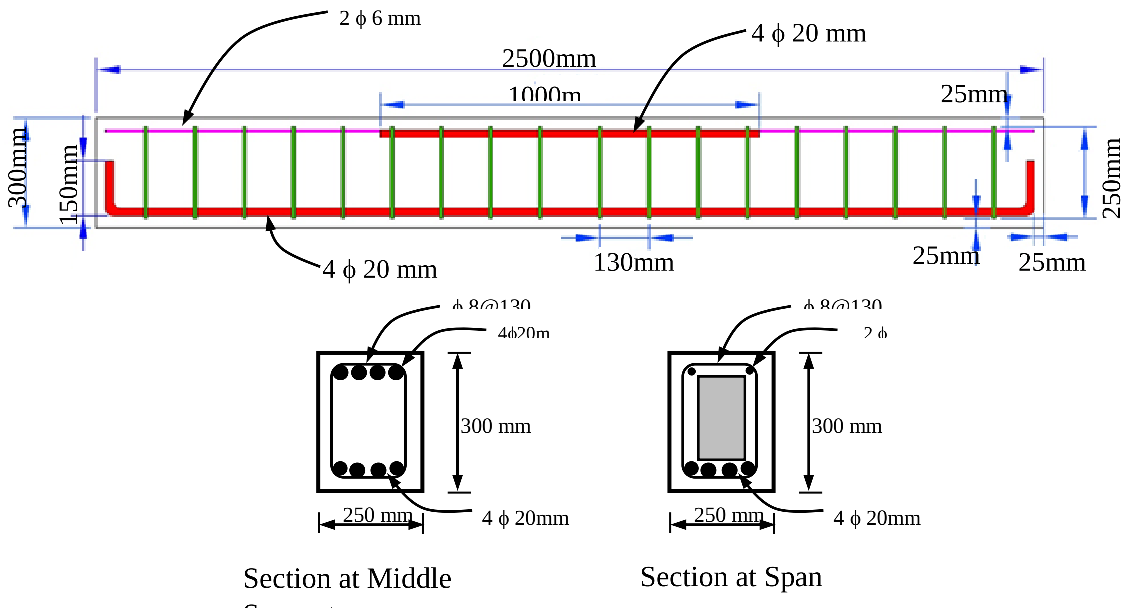

Figure (7) Reinforcement Details for Solid and Hollow Sections

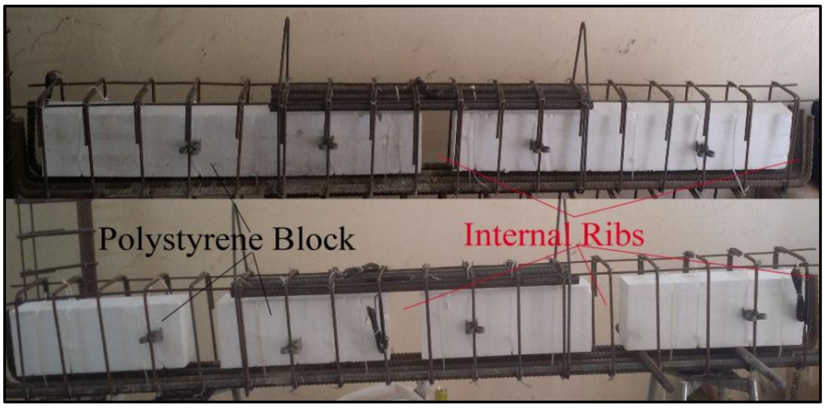

Figure (8) Using of Polystyrene Blocks to form Hollows in Beam Specimens

2-3-Materials

In manufacturing the beam specimens, the properties of the used materials are presented in Table (2); while, the concrete mix proportions are presented in Table (3).

Table (2) Properties of Construction Materials

Material	Descriptions
Cement	Ordinary Portland Cement (Type I)
Sand	Natural sand from Al-Ukhaider region with maximum size of (4.75mm)
Gravel	Crushed gravel of maximum size (12.5 mm)
Limestone powder	Fine limestone powder (locally named as Al-Gubra) of Jordanian origin
Superplasticizer	Glenium 51 manufactured by BASF Construction Chemicals, Jordan.
Reinforcing Bars	(ϕ20mm) deformed steel bar, having (612.74 MPa) yield strength (f_y) (ϕ8mm) deformed steel bar, having (761.95 MPa) yield strength (f_y) (ϕ6mm) plain steel bar, having (884.19 MPa) yield strength (f_y)
Water	Clean tap water

Table (3) Proportions of Concrete Mix

Material	Quantity
Cement (kg/m^3)	450
Fine Aggregate (kg/m^3)	778
Course Aggregate (kg/m^3)	890
Limestone Powder (kg/m^3)	50
Water (kg/m^3)	162.5
Water/Cement Ratio	0.36
Water/ Powder Ratio	0.325
Superplasticizer (L/m^3)	4.5
Superplasticizer/Cement Ratio	0.01

2-4-Test Measurements and Instrumentation

All beams were tested by using the Hydraulic Universal Testing Machine (MFL system) with a maximum range capacity of (3000kN). Vertical deflection was measured at mid-span and quarter span by using dial gauges of (0.01mm/div.) accuracy at every load stage. The gage is placed under the bottom face of the tested beams. The strains were measured by means of strain gauges attached at different locations as shown in Figure (9).

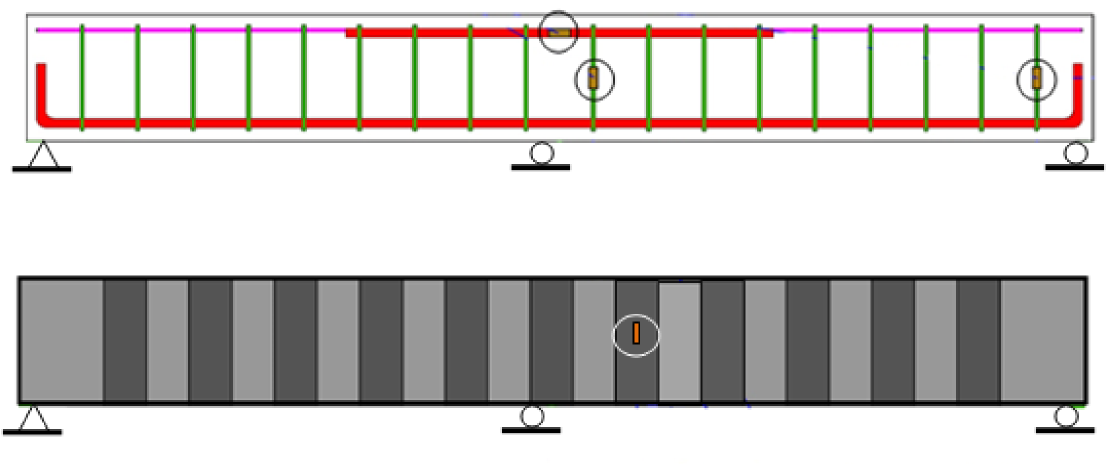

Figure (9) Strain Gauges Locations in Steel, Concrete and CFRP

2-5 Test Procedure

The beam specimens were tested at age of (28 days), where they were prepared by cleaning and painting them with white color, in order to detect the propagation of cracks. The beam specimens were placed on the testing machine with clear spans of (1210mm). Dial gauges were placed in their locations at mid span (at 605mm from the support) and quarter span (at 302.5mm form the middle support). The strain gauges were connected with a data logger (TML/ TC-32K). All beams were tested under monotonic loading, up to failure, with single concentrated load applied at the mid-span of each span of beams as shown in Figure (10).

Initially, each beam is loaded with a small load to ensure that the dial a gauge is in touch with the bottom faces of the beams and the strain gauges are working correctly. After that, the load was increased regularly at (1.0 kN/sec) and the readings taken every (10 kN). When the beams reached advanced stage of loading, smaller increments of load were applied until failure, as the load indicator stopped in recording or returned back and the deflection increased very fast without any increase in applied load. Throughout the test, all necessary measurements and observations were recorded.

Figure (10) Beam Specimen Setup

3-Results and Discussion

As mentioned before, the objective of this paper was to study the influence of the hollow core and strengthening with CFRP on the shear behavior of continuous reinforced concrete beams. During the experimental work, ultimate loads, load versus deflection and strains were recorded. Photographs for the tested beams are taken to show the crack pattern and some other details. The recorded data, general behavior and test observations are reported as well as recognizing the effects of various parameters on the shear behavior**.**

3-1 General Behavior

At early stages of loading, several cracks initiated near the middle support (at maximum shear), with further loading, these cracks appeared near the support edge and extended upwards, toward the load point, and became wider in shear spans. One or more cracks propagated faster than the others and reached the compression flange (near applied load), where crushing of the concrete near the positions of applied loads had occurred due to high concentrated stresses under load, Figure (11).

Figure (11) Crack Pattern

3-1 Failure Mechanism of SCC Beams

Shear failure compared with other failures is more devastating due to sudden failure. Shear failure starts from the critical section at high shear zone near the middle support. The failure is usually occurring without giving any alarming alerts. Therefore, shear failure is considered to be more dangerous for structures than flexural failure (Jumaat and Alam 2011). Diagonal cracks start from support to applied load and these diagonal cracks formed on one side or both sides together in RC beam and failure occurred by widen of shear cracks in RC beam (Täljsten 2003). The failure mode of the control beam was diagonal splitting failure on vertical plane, while the failure mode of the strengthened beams were either CFRP debonding or CFRP rupture, Figure (12) shows examples of the observed failure modes. For the beam specimens strengthened with U-shape wrap by CFRP, the debonding area was above the shear crack as shown in Figure (12-b).

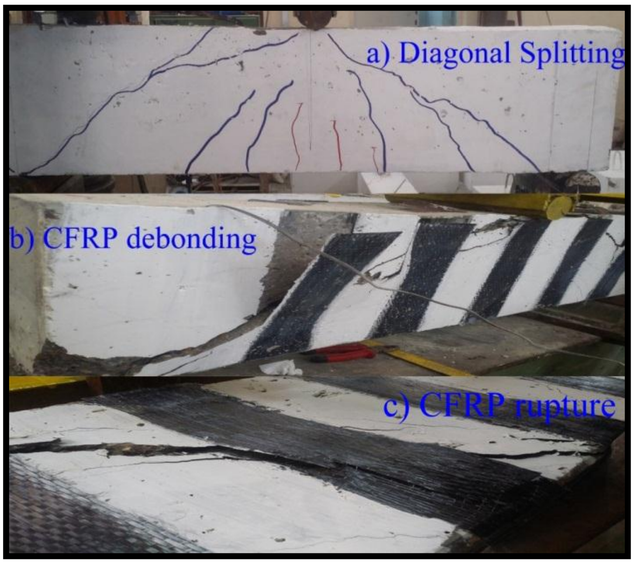

Figure (12) Mode of Failure of Tested Beams

Before failure of the beam specimens with warping U-shape CFRP, longitudinal cracks form on the top surface of the specimens. The crack initiated close to the position of applied load and extended towards the middle support as shown in Figure (13).

Figure (13) Cracks on the Top Surface of Beam Specimens

3-2 Ultimate Load

The ultimate load and the variation in the load capacity are presented in Table (4). The beam specimen (SCC-R0-00) featured one principal crack and finer cracks parallel to principal crack deployment of an average angle of (45 degrees) from the support to the load point, which is typical of continuous beam behavior. The ultimate load was attained when the principal crack extended deeper into the compression zone. The mode of failure was diagonal splitting failure with ultimate load of (915 kN), see Figure (12-a).

Table (4) Ultimate Load of Beam Specimens

Beam	Beam Encoding	Ultimate Load kN	(±) Load kN	(±) Load %	Mode of Failure
B1*	SCC-R0-00	915	0	0	diagonal splitting failure
B2	SCC-R3-00	635	-280	-30.6	diagonal splitting failure
B3	SCC-R5-00	735.5	-179.5	-19.62	diagonal splitting failure
B4	SCC-R0-90°	1090	175	19.13	CFRP debonding
B5	SCC-R3-90°	810	-105	-11.48	CFRP debonding
B6	SCC-R5-90°	1022.5	107.5	11.75	CFRP rupture

***Reference Beam**

The ultimate load for the (SCC-R3-00) was (635kN) and the mode of failure was diagonal splitting failure. The load level was reduced by (30.6%) compared with the reference beam. The ultimate load of beam specimen (SCC-R5-00) was (735.5kN) and the mode of failure is similar to (SCC-R3-00). The load level of (SCC-R5-00) was reduced by (19.62%) compared with reference beam. It may be noted that, the specimen (SCC-R5-00) has a higher level of load by (15.8%) compared with

(SCC-R3-00), this may be due to increasing of the number of internal ribs, which led to increasing the total volume of concrete; and the position of two internal ribs under the load(at mid span) cause this higher level. The influence of CFRP was clear in both, load and mode of failure. The beam specimen (SCC-R0-90º) failed at load of (1090 kN) and this is greater than by about (19.13%) compared with the reference. This may be due to high contribution of the CFRP, and as a result, the beam specimen failed due to CFRP debonding as shown in Figure (14).

The ultimate load for beam specimen (SCC-R3-90º) was (810 kN) and the mode of failure was CFRP debonding as shown in Figure (15). It may be noted that, the ultimate load of beam specimen (SCC-R3-90º) was decreased by about (11.48%) in comparison with the reference beam. This means that the CFRP strengthening reduces the damage of hollow beams but doesn't reach to the ultimate load of reference beam like the beams of five internal ribs. The gains in ultimate load due to strengthening by CFRP when compared beam specimen (SCC-R3-90º) with (SCC-R3-00) was (27.55%). The ultimate load for beam specimen (SCC-R5-90º) was (1022.5 kN) and the mode of failure was CFRP rupture as shown in Figure (16), The increase in ultimate load of the beam specimens (SCC-R5-90º) was (11.75%) in comparison with the reference beam. The increase of the ultimate load for the beam specimens (SCC-R5-90º) was (39%) in comparison with the beam specimen (SCC-R5-00). Clearly, this may be due to the contribution of strengthening by CFRP.

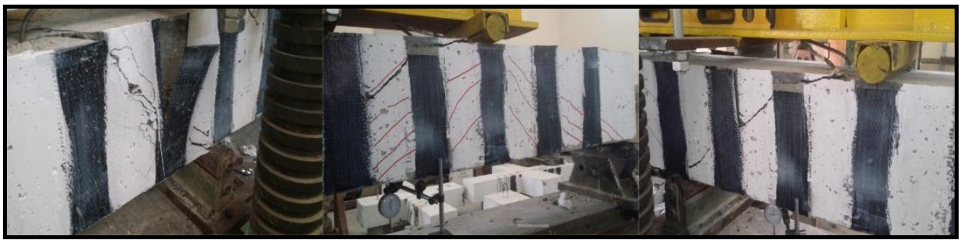

Figure (14): Debonding of CFRP for Beam Specimen (SCC-R0-90º)

Figure (15): Debonding of CFRP for Beam Specimen (SCC-R3-90º)

Figure (16): CFRP Rupture for Beam Specimen (SCC-R5-90º)

3-3 Load-Deflection Behavior

In all tested beams, the deflections (displacement in vertical direction) were measured at mid span and at quarter span (near the middle support). Load-displacement curves for the tested beams are plotted and presented in Figure (17). The result show that the hollow effect was observed clearly by reduced the defection, in mid span, for the beam specimen (SCC-R5-00), by (20.73%) and for the beam specimen (SCC-R3-00) by (22.68%) compared with reference beam. The effect CFRP on deflection for the solid beams was (129.27%) for beam specimen (SCC-R0-90) compared with reference beam. The results were deferent in the hollow section strengthened by CFRP, the deflection increased for beam specimens (SCC-R5-90º) by (47.56%) in comparison with reference beam. For beam specimen that have three internal ribs, the increase was (26.83%) for (SCC-R3-90º) in comparison with reference. The deflection was so large for the hollow beams strengthened by CFRP. After using of CFRP, the deflection was increased for about (64.65%) for beam specimens (SCC-R3-90º) in comparison with beam specimen (SCC-R3-00), and for beam specimens (SCC-R5-90), the increase was (86.15%) compared with beam specimen (SCC-R5-00). The effect of the number of internal ribs was observed when comparing the deflection of beam specimens (SCC-R3-00) and (SCC-R3-90º) with (SCC-R5-00) and (SCC-R5-90º) in which the increases in deflection were about (2.52%), (16.34%) respectively.

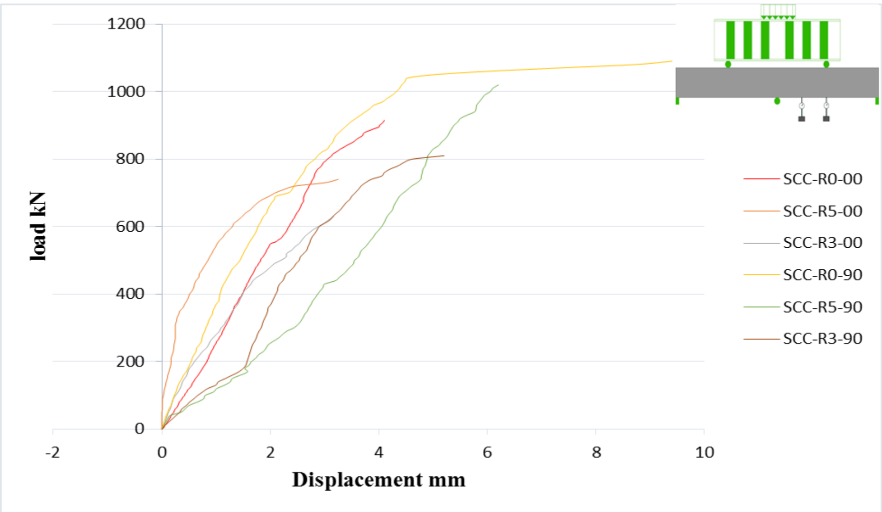

Figure (17) Load-Deflection Relationship at Mid Span

3-4 Load-Strain Behavior

The instrumentations for strain monitoring were carefully designed to provide the information and data much needed for the understanding of the shear resistance mechanisms involved in beams retrofitted with FRP. It must be realized that all the recorded data was subjected to careful examination, analysis, and comparisons.

3-4-1 Concrete Strain

According to ACI-318-14, the maximum compressive strain of concrete at crushing was (0.003) to higher than (0.008) under special conditions. However, the strain at which ultimate moments are developed is usually about (0.003) to (0.004) for members of normal proportions and materials. In the present paper, the value of strain was recorded every (10 kN), Figure (18) shows the curves representing the shear force in the lateral direction of concrete at distance of (303mm) from the middle support. For the beam specimen (SCC-R3-00), the mode of failure was a diagonal splitting failure, this means the ultimate load exceeds the ultimate stress and the concrete approaches to peak response. The other two beam specimens (SCC-R5-90º) and (SCC-R3-90º) have the same CFRP rupture failure which means that the CFRP is look like the jacket working to restrict the concrete, and as a result, the stresses and strains were increased. Before failure of CFRP, the strain in concrete was increase slowly but at the CFRP, the stain was jumped over the strain of concrete. Figure (18B) show the load-strain curves for concrete at mid span. Maximum bending strain in all beams was less than the ultimate strain of concrete except for beam specimen (SCC-R5-90º). For solid beams, the strains were closed and the solid beams strengthened by CFRP give the higher values than the solid without strengthening. The increasing in strains for beam specimen (SCC-R0-90º) was (23.69%) compared with (SCC-R0-00).

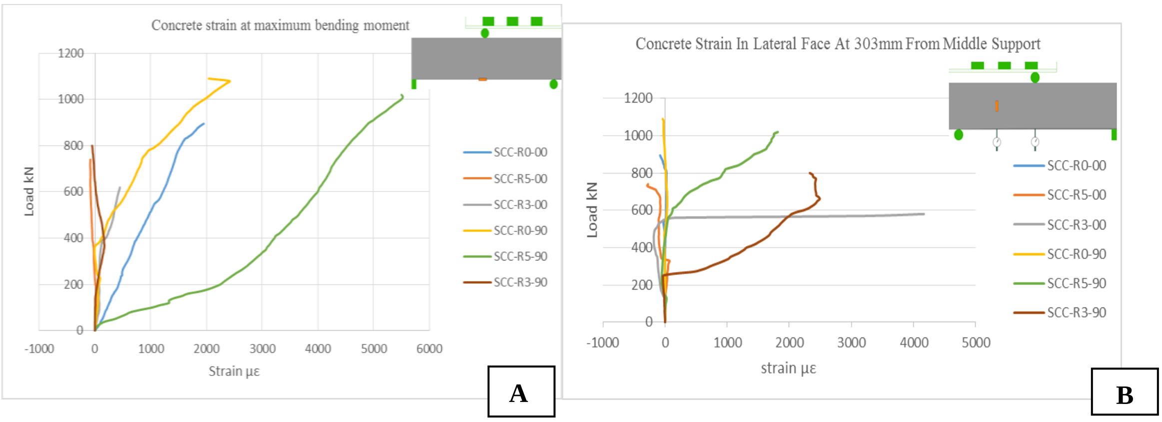

Figure (18) Strain in Concrete at Quarter and Mid Span (Microstrain)

3-4-2 Steel Reinforcement Strains

The yield and ultimate strains for shear reinforcement (stirrups) were calculated from stresses assuming (Es=200x10^3 MPa). The yield and the ultimate strains were (0.0038) and (0.0048) respectively, Figures (19-a) and (19-b). The strain gauges were fixed on stirrups at distance of (d/2) from the middle support and on stirrups at distance of (91mm) from the edge support. Experimental results indicated that the measured strains didn't reach the yield strain in steel reinforcement. The strain for stirrups at critical section (d/2) from the middle support for all beams did not reach to yield strain. The strains in beam specimens (SCC-R3-00º) and (SCC-R5-90º) give higher values compared with the other beams. The strains in beam specimen (SCC-R3-00º) were higher than for (SCC-R5-90º). Solid beam specimens without strengthening give less value relative to other beams and the beams with five internal ribs without strengthening give less value than the beams with three internal ribs. The strain in the stirrups at edge was less than the strain at distance of (d/2) form the middle support because the shear value at the edge support was half value of the shear at the middle support. For the top steel reinforcement, the yield strain was (0.00306) and ultimate strain was (0.00365). Figure (20) shows the strains for SCC beams at maximum negative moment. All measured strains didn't reach the yield strain of steel bar. The solid beam specimens strengthened by CFRP give higher strain values compared with the others. The maximum strain for beam specimen (SSC-R0-90º) occure at the load of (1020 kN) which represents (93.5%) of the ultimate load. For the strengthened beam specimens that failed by debonding of CFRP, the strain values were higher than the values of the beams failed by rupture. CFRP strips reduce the growth of diagonal cracks and reduce their progression into the compression zone. When the debonding takes place, more cracks appeared in the tensile zone of concrete which is exposed to maximum moment.

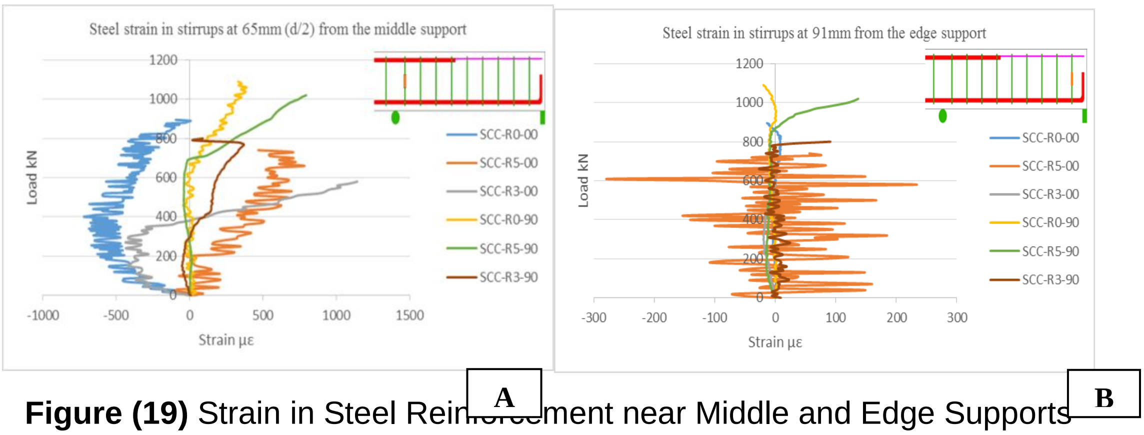

Figure (19) Strain in Steel Reinforcement near Middle and Edge Supports (Microstrain)

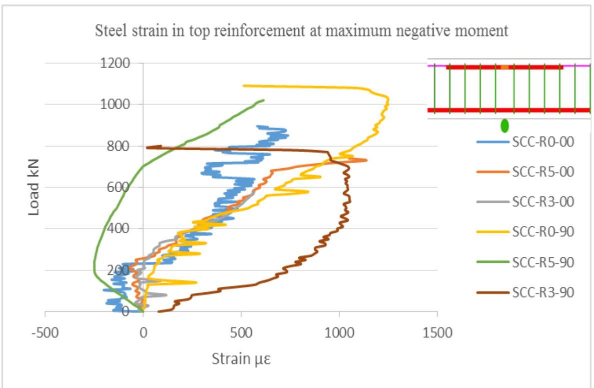

Figure (20) Strain in Steel Reinforcement at Maximum Negative Moment (Microstrain)

3-4-3 CFRP Strain

Figure (21) shows the load-strain curves in the CFRP for strengthened SCC beam specimens. After the first crack, most of stress shift from the concrete to steel reinforcement and CFRP in the advanced stages of loading. The evident through the relationship between strain and load show that the strain in beams with CFRP strips was large in the beam that contains less amount of concrete.

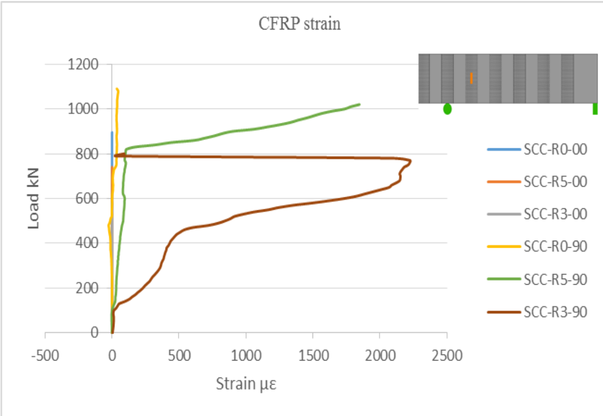

Figure (21) Strain in CFRP at First Strips from Middle Span (Microstrain)

3-5 Effect of Beam Weight

The hollows in the beams means that there are two basic characteristics, the first one is advantage in reducing the weight of the beam and the second one is a disadvantage because it reduces the failure load of the beam. Therefore the (CFRP) were used to reduce the disadvantage of the hollow. The gain in the dead weight of the beam reduces the ultimate load by about (19.62-30.6%) compared with solid beam without strengthening, while when strengthening with CFRP the ultimate load increased by (11.7%) for the (90º) orientation of the CFRP strips. This means the goal was achieved in overcoming on the disadvantage of the hollows by strengthening the beam by CFRP. For beam specimens poured with three ribs and strengthening by CFRP, the goal to reach the ultimate load of the solid beam (reference beam) was not achieved but that not means the strengthening was unsuccessful. The reduction in ultimate load was (11.47%) for the (90º) orientation of the CFRP. Table (5) shows the effect of hollow to reduce the ultimate load.

Table (5) Load Due to Variation in Weight

Beam Encoding	Ultimate Load (kN)	Weight (kN)	Variation in Weight	% of Hollow
SCC-R0-00	915	4.58	0%	0%
SCC-R0-90º	1090			
SCC-R5-00	735.5	3.85	16%	16%
SCC-R5-90º	1022.5			
SCC-R3-00	635	3.78	17.6%	17.6%
SCC-R3-90º	810			

4- Conclusions

Based on the obtained results, observations and discussion, the following conclusions can be drawn:

1-The ultimate (failure) load for the hollow beams strengthened by five and three internal ribs reduced by about (19.62%) and (30.6%) respectively compared with reference beam. The CFRP effects on failure load by increasing the load capacity for about (19.13%) for solid beams compared with the reference beam.

2- The number and location of internal ribs affects significantly on the ultimate load. The beam specimens which have five internal ribs give (15.8%) higher than ultimate load the beam specimens which have three internal ribs. This may be due to contribution of the additional internal ribs under the concentrated load.

3- The ultimate load was increased by about (39%) for five internal ribs hollow beams strengthened by CFRP strips of (90º) orientation, compared with the hollow beam with five internal ribs without strengthening. The ultimate load was increased (11.74%) for (90º) CFRP orientation after there being lost because of hollows by (19.13%) in compared with the reference beam.

4- The ultimate load was increased for about (27.5%) for beam specimen which has three internal ribs with CFRP strips of (90º) orientation, in comparison with the hollow beam that has three internal ribs without strengthening; and this led to reduce the losses in load capacity from the (30.6%) to (11.47%) for (90º) orientation.

5- Reference

1-ACI-318 (2014). "*Building Code Requirements for Structural Concrete (ACI-318M-14) and Commentary (ACI-318RM-14)*", American Concrete Institute.

2-Ahmed Khalifa, Gustavo Tumialan, Antonio Nanni, and Abdeldjelil Belarbi (1999). "*Shear Strengthening Of Continuous RC Beams Using Externally Bonded CFRP Sheets*". Journal of SP-188, American Concrete Institute, Proc., 4th International Symposium on FRP for Reinforcement of Concrete Structures (FRPRCS4), Baltimore, MD, Nov. 1999. Pp 1-15

3- Täljsten, B. "*Strengthening Concrete beams for shear with CFRP sheets*", Construction and Building Materials, vol. 17, pp. 15-26, 2003.

4-Bakis, C. E.; Bank, L. C.; Brown, V. L.; Cosenza, E.; Davalos, J. F.; Lesko, J. J.; Machida, A.; Rizkalla, S. H.; and Triantafillou, T. C., 2002, "*Fiber- Reinforced Polymer Composites for Construction-State-of-the-Art Review*", Journal of Composites for Construction, V. 6, No. 2, pp. 73-187.

5-Clarke, J. L., 2000, "*The Use of Fiber Composites in Concrete Bridges: A State of the Art Review*", Technical Guide No. 3, Concrete Bridge Development Group, 36 pp.

6-. Jumaat M. Z and Alam, M. A. "*Optimization of intermediate anchors to eliminate premature shear failure of CFRP laminate flexurally strengthened RC beams*", Int. J. Phys. Sci, vol. 6, pp. 182-192, 2011.

7-Neale, K. W., 2000, "*FRPs for Structural Rehabilitation: A Survey of Recent Progress*", Progress in Structural Engineering and Materials, V. 2, pp. 133-138.

8-Täljsten B, Elfgren L. "*Strengthening concrete beams for shear using CFRP materials: evaluation of different application methods*",. Composites: Part B 2000; 31:87–96.

Ali Hameed Aziz Ph.D (Asst.Prof.)* Ashraf A. Alfeehan Ph.D (Asst.Prof.)* awaz Adel Hussain(lecturer)*

استخدام شرائح الكاربون اللدائنية المسلحة لتقوية القص للعتبات الخرسانية ذاتية الرص-المستمرة المجوفة والحاوية على اضلاع داخلية مستعرضة

أ.م.د.علي حميد عزيز* أ.م.د.اشرف عبد الهادي الفيحان* م.م. فواز عادل حسين

المستخلص

يختص هذا البحث بالتحري العملي لمقاومة القص للعتبات الخرسانية ذاتية الرص،المستمرة ،المجوفة والحاوية على اضلاع داخلية مستعرضة والمقواة خارجيا باستخدام شرائح ألياف الكاربون اللدائنية المسلحة (CFRP). ست عتبات بمقطع حقيقي تم صبها وفحصها في هذا البحث. تم اعتماد متغيرين في هذه الدراسة وهما عدد الأضلاع الخرسانية الداخلية المستعرضة وشرائح التقوية بألياف الكاربون المثبتة بإتجاه عمودي على جانبي مقطع العتبة. اظهرت النتائج العملية أن الفشل بالقص هو الفشل السائد لجميع العتبات التي تم اختبارها. حدث نقصان في حمل التشقق و الحمل الاقصى بحدود (19.6-30.6%) للعتبات المجوفة الحاوية على خمسة وثلاثة اضلاع على التوالي مقارنة مع العتبة المرجعية. ازداد الحمل الاقصى بحدود (19.3%) للعتبات الصلدة المقواة بشرائح ألياف الكربون اللدائنية مقارنة مع نفس العتبة غير المقواة .كذلك، ازداد الحمل الاقصى بحدود (39%) للعتبات المجوفة المقواة بشرائح ألياف الكربون اللدائنية الحاوية على خمسة اضلاع داخلية مقارنة مع نفس العتبة غير المقواة, بينما ازداد الحمل الاقصى بحدود (27.5%) للعتبات المجوفة المقواة بشرائح ألياف الكربون اللدائنية الحاوية على ثلاثة اضلاع داخلية مقارنة مع نفس العتبة غير المقواة

*الجامعة المستنصرية